

AD-A145 958

NATIONAL PROGRAM FOR INSPECTION OF NON-FEDERAL DAMS
LAKE ELLIS DAM MA 000 (U) CORPS OF ENGINEERS WALTHAM
MA NEW ENGLAND DIV APR 81

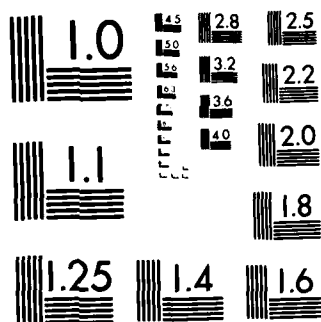
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MICROCOPY RESOLUTION TEST CHART
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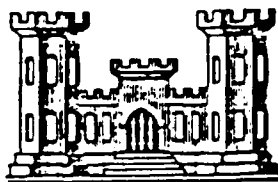
AD-A145 958

CONNECTICUT RIVER BASIN
ATHOL, MASSACHUSETTS

LAKE ELLIS DAM
MA 00005

PHASE I INSPECTION REPORT
NATIONAL DAM INSPECTION PROGRAM

DTIC FILE COPY



DEPARTMENT OF THE ARMY
NEW ENGLAND DIVISION, CORPS OF ENGINEERS
WALTHAM, MASS. 02154

APRIL 1981

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UNCLASSIFIED

SECURITY CLASSIFICATION OF THIS PAGE (When Data Entered)

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9. PERFORMING ORGANIZATION NAME AND ADDRESS		8. CONTRACT OR GRANT NUMBER(s)
11. CONTROLLING OFFICE NAME AND ADDRESS DEPT. OF THE ARMY, CORPS OF ENGINEERS NEW ENGLAND DIVISION, NEDED 424 TRAPELO ROAD, WALTHAM, MA. 02254		10. PROGRAM ELEMENT, PROJECT, TASK AREA & WORK UNIT NUMBERS
14. MONITORING AGENCY NAME & ADDRESS (if different from Controlling Office)		12. REPORT DATE April 1981
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19. KEY WORDS (Continue on reverse side if necessary and identify by block number) DAMS, INSPECTION, DAM SAFETY, Connecticut River Basin Athol, Massachusetts		
20. ABSTRACT (Continue on reverse side if necessary and identify by block number) Lake Ellis Dam is an earth embankment about 140 feet long with a maximum height of about 11 feet. The dam is in fair condition. The culverts under the highway are capable of discharging 2,400 cfs which is equivalent to 77% of the PMF before the highway embankment would be overtopped. The full PMF event would overtop the highway embankment by approximately one foot.		

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Justification	
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A-1	



DESCRIPTION

Identification Number:	MA 00005
Name of Dam:	Lake Ellis Dam
Town:	Athol
County and State:	Worcester County, Massachusetts
Stream:	Mill Brook
Date of Inspection:	December 4, 1980

Lake Ellis Dam is an earth embankment about 140 feet long with a maximum height of about 11 feet. Water Street, a hard surfaced road, traverses the crest of the dam which is approximately 20 feet wide. Both the upstream and downstream slopes of the embankment are about 1.5H:1V with trees and brush covering most of the surface. The masonry overflow spillway, which has been blocked off since 1948, never experienced discharge in its 59 year history. All discharges in excess of normal flow from Lake Ellis are directed into South Brook on the south side of the watershed. The Massachusetts Route 2 embankment acts as the dam for the watershed where it crosses the marsh leading to South Brook.

The 27-inch diameter penstock, which used to provide water for the razed Swift River Box Company, is the only source of water to Mill Brook from the watershed above Lake Ellis Dam.

The impoundment is currently used for recreational purposes only.

The general condition of the dam appears to be fair.

EVALUATION OF HYDRAULIC/HYDROLOGICAL FEATURES

Lake Ellis Dam has a watershed of 3.0 square miles which is better than 90 percent forest covered. The remainder of the watershed is residentially and commercially developed. The terrain is relatively steep ranging from El. 1,388 on the eastern edge of the watershed to El. 832 at normal pool. Lake Ellis Dam does not have a spillway. The storage capacity with the reservoir level at the top of the dam El. 839 is 970 Acre-Feet. On the south side of the watershed the Massachusetts Route 2 embankment, which is 8 feet high with a crest width of approximately 60 feet and side slopes about 4H:1V, acts as the dam for the watershed. The spillway system through the highway embankment consists of three 16-foot wide by 5-foot high culverts.

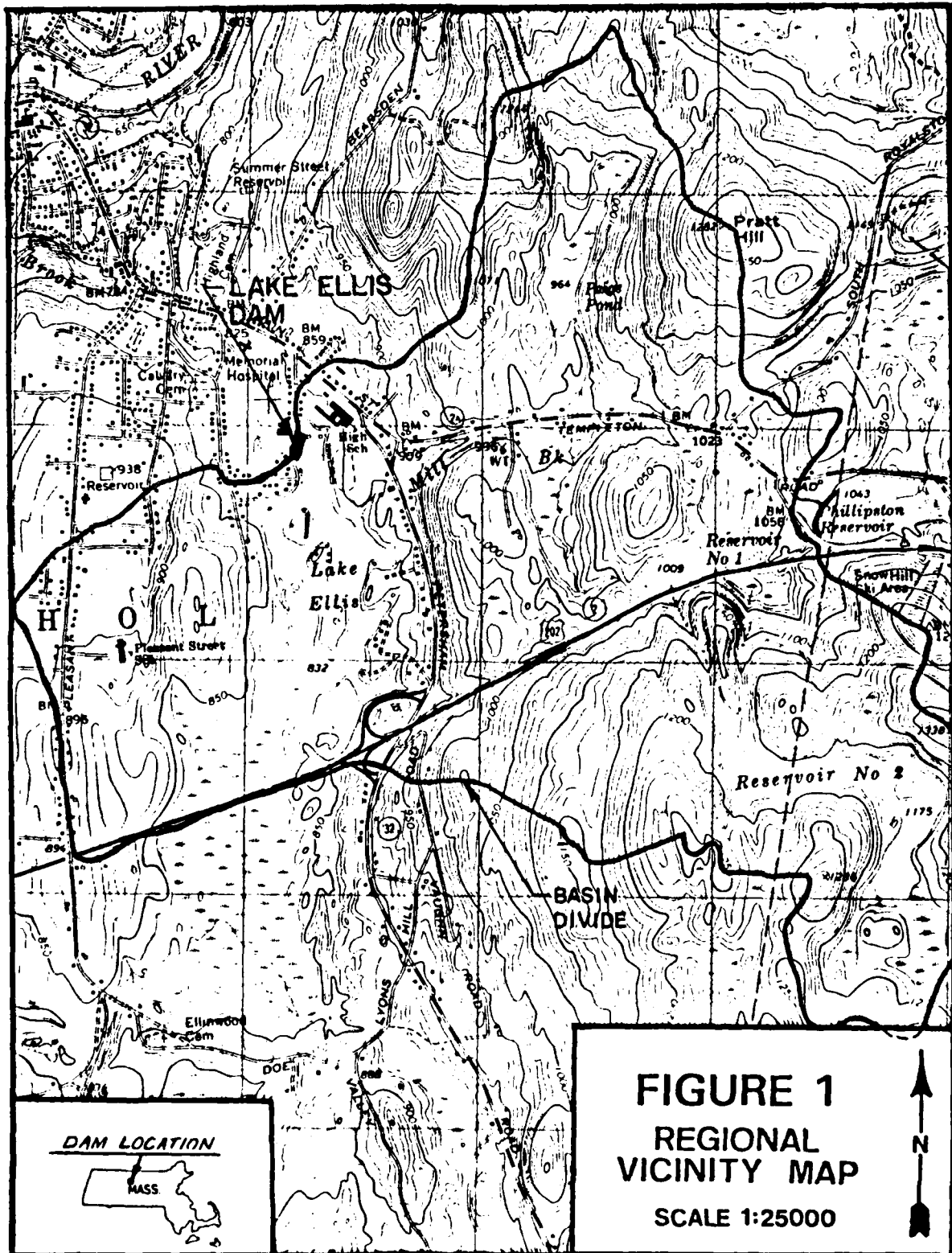
The culverts under the highway are capable of discharging 2,400 cfs which is equivalent to 77 percent of the PMF before the highway embankment would be overtopped. The full PMF event would overtop the highway embankment by approximately one foot.



UPSTREAM OVERVIEW OF THE DAM FROM THE SOUTH ABUTMENT.



DOWNSTREAM OVERVIEW OF THE DAM FROM THE SOUTH ABUTMENT.



APPENDIX **A**
ENGINEERING DATA

*From Lord's "History of Athol" supplied by James
Tedford of Athol.*

Mill Brook

We have no record or tradition as to when or why Mill Brook was so named. It is mentioned in the record of the second contract made by the proprietors with Samuel Kendall at which time so far as we know no power had been developed there, except the saw mill on Freedom Street which originally was purely a Mill Brook power as the canal from the river into its pond was not excavated until some twenty years or more later.

In relating the story of the mills along this brook and the other streams we will (as we have done in the story of South Brook) tell the story in the order of their location, beginning at the head waters rather than by dates of development.

About the middle of the nineteenth century industrial development along Mill Brook made a more steady flow of its waters imperative, and to accomplish this a group of these mill owners planned a series of reservoirs to impound flood waters and release them in periods of drought. This original group consisted of Abram and Ira Oakes, owners at Pegville, Edwin Ellis, owner at Water Street, Festus F. Amsden, then interested at Mechanic Street, Jacob S. Cooke and Laban Morse at Pleasant Street, and Dexter Cheney, an adjoining owner, Theodore Jones at Mechanic Place, Frederick Jones and Milton Baker at Chestnut and Main Streets, and Lyman W. Hapgood at Chestnut and Hapgood Streets.

The first reservoir built by these associates was in Athol, a short half mile south of our former Welfare Home; the next and larger reservoir, known as No. 2, was perhaps a quarter of a mile south of No. 1, its dam being in Athol but much of its flowage area in Phillipston. As this is written, studies are being made looking to the taking of this No. 2 reservoir for an additional source of water for Athol's Municipal Water System.

No. 3 reservoir was built about 1854, on a small brook tributary to Mill Brook and a short half mile northeast of Pegville.

Some time later these associates were incorporated as Athol Water Company, thus forcing Robert and Solon Wiley to adopt another name for their original municipal water system. They chose Athol Aqueduct Company as their name but at a later period Athol Water Company became Athol Reservoir Company and still later Mill Brook Associates and Athol Aqueduct Company became Athol Water Company.

Downstream from these reservoirs the first industrial plant we come to was at "Pegville", the dam being west of Garfield Road and the mill pond extending well east of that road. Here long ago was a flourishing mill producing a variety of wooden items including shoe pegs. It was operated by Abraham Oakes and later by his son Ira, one George Wilder, and for a short time L. J. Whitney. In early deeds, Abraham Oakes describes himself as a "plough maker" which is not inconsistent with the operating of a woodworking plant as these implements were made entirely of wood previous to around 1835. According to my notes this mill was first taxed in 1833.

When the use of wooden pegs in the manufacture of foot-wear ceased, this property fell into disuse and was eventually acquired by Athol Water Company and a mechanical water filter installed there. Since the construction of the filter beds off the north end of Hillside Terrace, this power has been entirely idle although the dam is maintained by the Water Department.

Next below this filter location is a substantial stone dam said to have been built by the Pegville owners when their business seemed to warrant expansion, but my information is that the project was never completed.

Next on the stream and just east of Petersham Road was a substantial reservoir built as an auxiliary to the saw mill below.

The reserve waters from this pond were released into a small fore-bay west of Petersham Road, thereby wasting fully half the available "head" for this mill. Various owners recalled are Edward Drury, William B. Spooner, Orcutt and Samuel D. Prouty, Jonathan Drury, Eric D. Walker, Edmund Moore, Charles H. Butterworth, and T. Sidney Mann the present owner. The land and buildings are utilized by the Mann Lumber Company, but the use of the water power was discontinued some twenty years ago.

Pressing hard upon the tail race of this last named saw mill are the waters of Lake Ellis, the most pretentious development on Mill Brook.

Much of the land covered by these waters was, previous to about 1840, a peat bog from which that fuel was cut at times.



EDWIN ELLIS FACTORY, WATER STREET, ABOUT 1885

Across this bog the road to Templeton and Boston crossed when first laid out in 1754, this section being a cordroy or log road.

June 22, 1843, Timothy Hoar, William Fletcher and Jonathan Kidder acquired some fifteen acres of land which later became a part of the Ellis plant. At approximately where Water Street crosses Mill Brook they built a dam of rather large proportions utilizing the water to produce power for a woodworking mill which they erected west of the dam.

There is some evidence that this mill power was a rejuvenation of a mill power long before developed and abandoned on this site, but the mill pond dates back only to 1843. The entire area was a part of a hundred acre lot (126 acres) laid out to John Ballard on the right of John Fiske and is numbered 47 on the plan.

Kidder, early in 1845, disposed of his share in this enterprise to his partners, and the preponderance of evidence is that later in 1845 Mr. Fletcher likewise retired from the business.

On December 18, 1845, this dam suddenly gave way releasing a large volume of water into Mill Brook. The Hoar factory was destroyed, as were several others down the stream.

Succeeding this disaster, the ownership passed through several hands until finally on March 13, 1852, it became the property of Edwin Ellis, a native of (North) Orange who speedily developed it into a prosperous industry continuing until his death on July 9, 1888. Succeeding him, his family carried on for a time.

In 1889 the Athol Reservoir Company entered upon a plan to improve the water powers of Mill Brook by increasing many fold the storage capacity of Lake Ellis.

It acquired the old Ellinwood dam and power on Doe Valley Road, got assents to flow intervening land, and asked the town of Athol, in the guise of improving Water Street, to rebuild the Ellis dam raising its spillway three feet, giving assurance that if this could be done the reservoir company would attend to all other details including land damages and the building of a new dam at Doe Valley Road with the spillway the same height as that contemplated at Lake Ellis. Against the better judgment of many citizens the appropriation was made and the prosecution of the work delegated to Gardiner Lord, Orrin F. Hunt and Henry Grey, its Selectmen, with the addition to the committee of James M. Lee, O. A. Fay and James F. Whitcomb.

The Water Street job was completed in 1889 as directed by the town, the spillway was raised the full three feet and an iron bridge constructed carrying Water Street across the dam, but none of the waters of Lake Ellis have ever flowed over that spillway for the agreement of Athol Reservoir Company was not kept and no dam was built at Doe Valley Road.

The bridge at Water Street was maintained until 1948 when it was removed and an earthen roadway constructed above the spillway.

The 1889 construction provided for a pen stock into the Ellis Plant and through this for sixty years a moderate supply of water has been released into Mill Brook, but all surplus or flood waters have found their way into South Brook and through it into White Pond, Rohunta and Millers River.

Adverse economic conditions eventually proved to be the undoing of the Ellis family. To add to their trouble the plant was destroyed by fire on May 15, 1896, but was rebuilt and was again in operation in less than a year. Bankruptcy proceedings ensued in late 1897 and the plant was sold at auction to Alfred J. Raymond and Millard W. White, both of Royalston; Mr. Raymond also purchasing the Ellis homestead at 1405 Main Street.

Mr. White soon retired from the business leaving Mr. Raymond as sole owner and he carried on an active business there for some thirty years.

Eventually financial difficulties overtook Mr. Raymond also and the plant again went under the auctioneer's hammer.

After a relatively short period of idleness it was acquired by Swift River Box Company, a refugee from North Dana which village was wiped out by the huge Quabbin Reservoir project and by this company active operations are still continued there but it is many years since the water power at this plant has been utilized.

INSPECTION REPORT & DATA FOR DAMS Jan 1972

Owner: Swift River Box Co.
 His Address: _____
 Function of Dam: Recreation

Location & Access: On Lake Ellis Rd 0.2 miles
So of Main St.

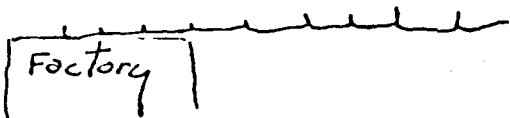
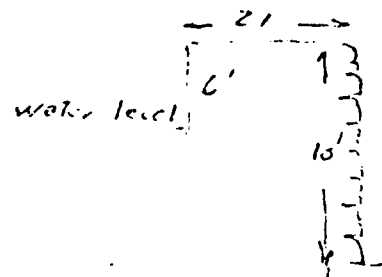
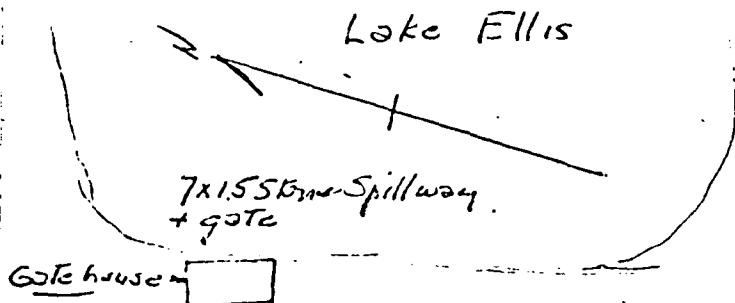
USGS Quad. Athol Lat. 42°34'50" Long. 72°12'20"
 Drain. Ar. 432 Sq. Mi. Ponds: 52 ac.; Res. @ dam: _____
 Character of D.A.: _____

Estimated 1.67
 Discharge: _____
 Capacity: _____

General Description of Dam and Discharge Control:

Earth filled stone faced on down stream side
Gate house (locked) maintenance man says gates probably
don't work

Sketch (Not to Scale):



Remarks and Recommendations:

Small leaks noted. Trees + brush on dam should be cut.
Water level maintained 3' ± below small spillway. Only outlet
presently used goes through pipe in boiler room probably
broken

Date 1-4-72 By Eaton & Gony Comment _____

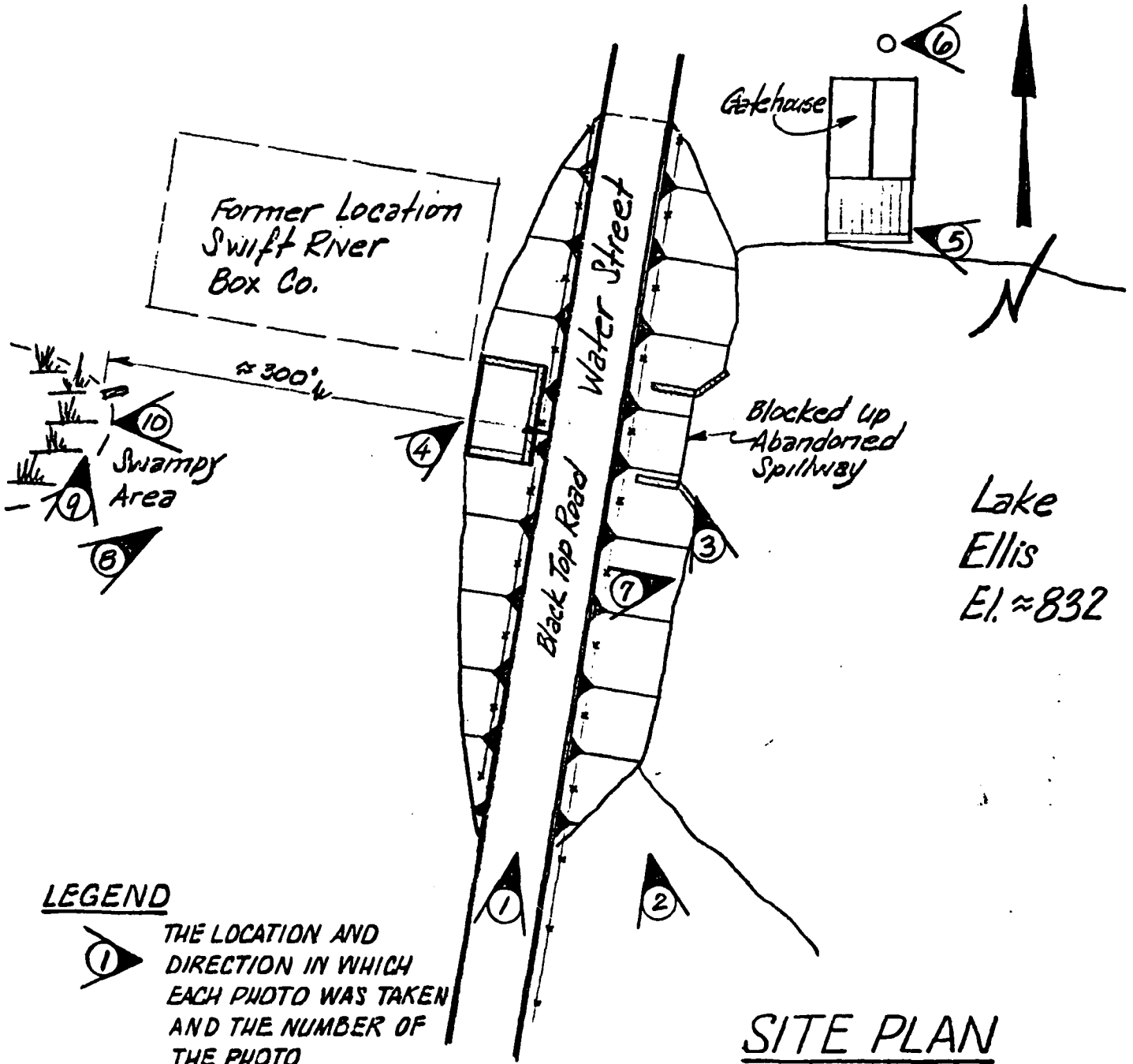
Dam No. 3-14-15-13

APPENDIX B
PHOTOGRAPHS

APPENDIX **B**
SELECTED PHOTOGRAPHS OF THE PROJECT

	<u>Page No.</u>
Site Plan	A
<u>PHOTOGRAPHS</u>	
<u>No.</u>	
1. View along the crest of the dam from the south abutment. (12/4/80)	1
2. Upstream face of the dam showing tree and brush cover. (12/4/80)	1
3. Inlet to spillway which has been blocked off. (12/4/80)	2
4. Downstream side of the spillway which has been blocked off. (12/4/80)	2
5. Recent construction in the vicinity of the gatehouse. (12/4/80)	3
6. Conduit for valve stem near gatehouse. (12/4/80)	3
7. Overview of Ellis Lake as seen from the dam. (12/4/80)	4
8. Former outlet channel of the former spillway. (12/4/80)	4
9. Outlet of 27-inch metal pipe about 300 feet downstream of the dam. (12/4/80)	5
10. Typical channel conditions downstream of the 27-inch metal pipe outlet. (12/4/80)	5

SUBJECT	SHEET	BY	DATE	JOB NO.
Lake Ellis Dam	A			





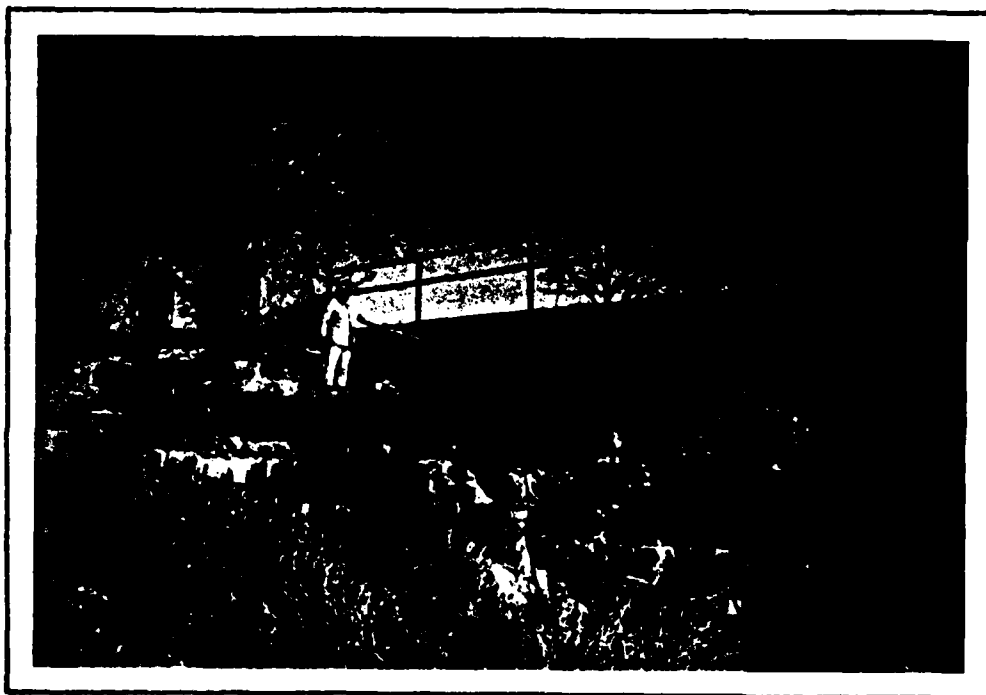
1. VIEW ALONG THE CREST OF THE DAM FROM THE SOUTH ABUTMENT. (12/4/80)



2. UPSTREAM FACE OF THE DAM SHOWING TREE AND BRUSH COVER. (12/4/80)



3. INLET TO SPILLWAY WHICH HAS BEEN BLOCKED OFF. (12/4/80)



4. DOWNSTREAM SIDE OF THE SPILLWAY WHICH HAS BEEN BLOCKED OFF. (12/4/80)



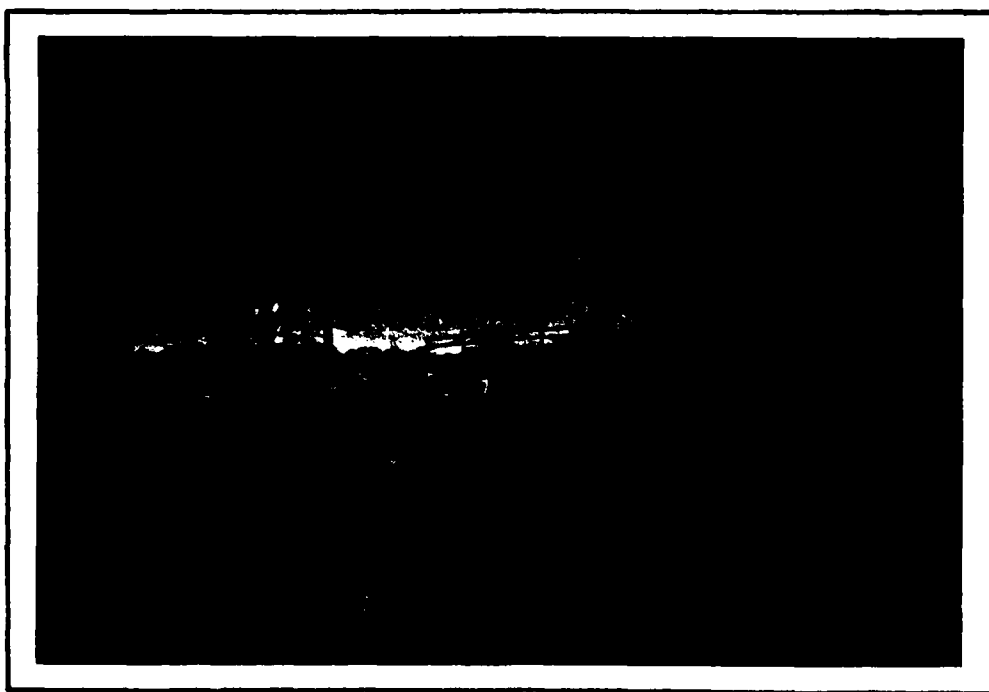
5. RECENT CONSTRUCTION IN THE VICINITY OF THE GATEHOUSE.
(12/4/80)



6. CONDUIT FOR VALVE STEM NEAR GATEHOUSE. (12/4/80)



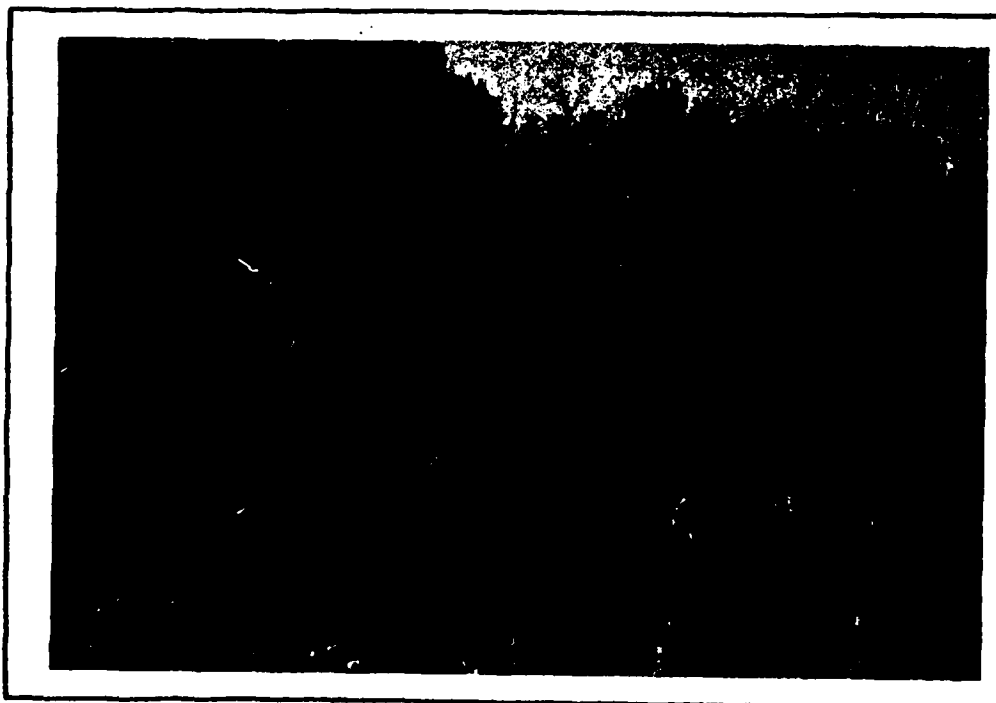
7. OVERVIEW OF ELLIS LAKE AS SEEN FROM THE DAM. (12/4/80)



8. FORMER OUTLET CHANNEL OF THE FORMER SPILLWAY. (12/4/80)



9. OUTLET OF 27-INCH METAL PIPE ABOUT 300 FEET DOWNSTREAM OF THE DAM. (12/4/80)



10. TYPICAL CHANNEL CONDITIONS DOWNSTREAM OF THE 27-INCH METAL PIPE OUTLET.

APPENDIX C
HYDROLOGIC AND HYDRAULIC COMPUTATIONS



SUBJECT

Lake Ellis

SHEET

1

BY

SK

DATE

2/24/81

JOB NO

2060.002

Hydrologic & Hydraulic ComputationsDrainage Area = 3.0 mi²Reservoir Area - From USGS Quad.Area (Acres)Storage* (AF)Assume Bottom of Impoundment
≈ El. 820

0

0

Normal Reservoir Surface ≈ El. 832

66

264

Crest Lake Ellis Dam & Crest R. 2 Emb. ≈ El. 839

140

970

El. 840

152

1115

Snyder Hydrograph Coefficients* Storage Formula $S = H/3 (A_1 + A_2 + \sqrt{A_1 A_2})$ H = incremental height
S = " storage $C_x = 2.0, C_p = 0.6$ $L = 3.7 \text{ miles}, L_{ca} = 1.9 \text{ miles}$

$$\therefore T_p = C_x (L \cdot L_{ca})^{0.3}$$

$$= 2 (3.7 \times 1.9)^{0.3}$$

 $T_p = 3.6 \text{ hrs.}$ PMP DataFrom HMR #33, 24 hr. 200 mi² = 20.5"Distribution

6 hr. 111%

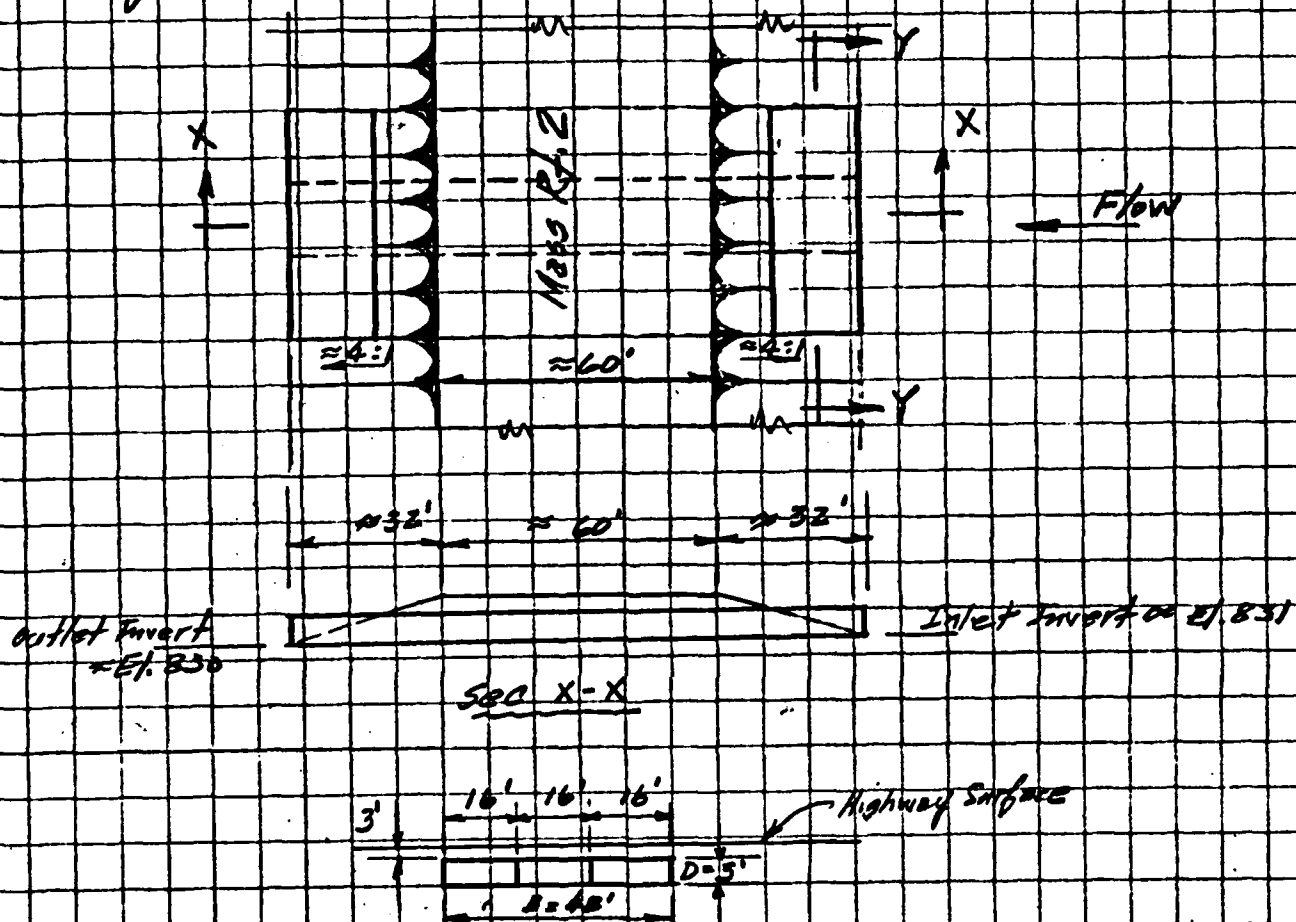
12 hr. 123

24 hr. 133

48 hr. 142

SUBJECT	DATE	BY	JOB NO.
Lake Ellis	2/24/21	JS	2060.002

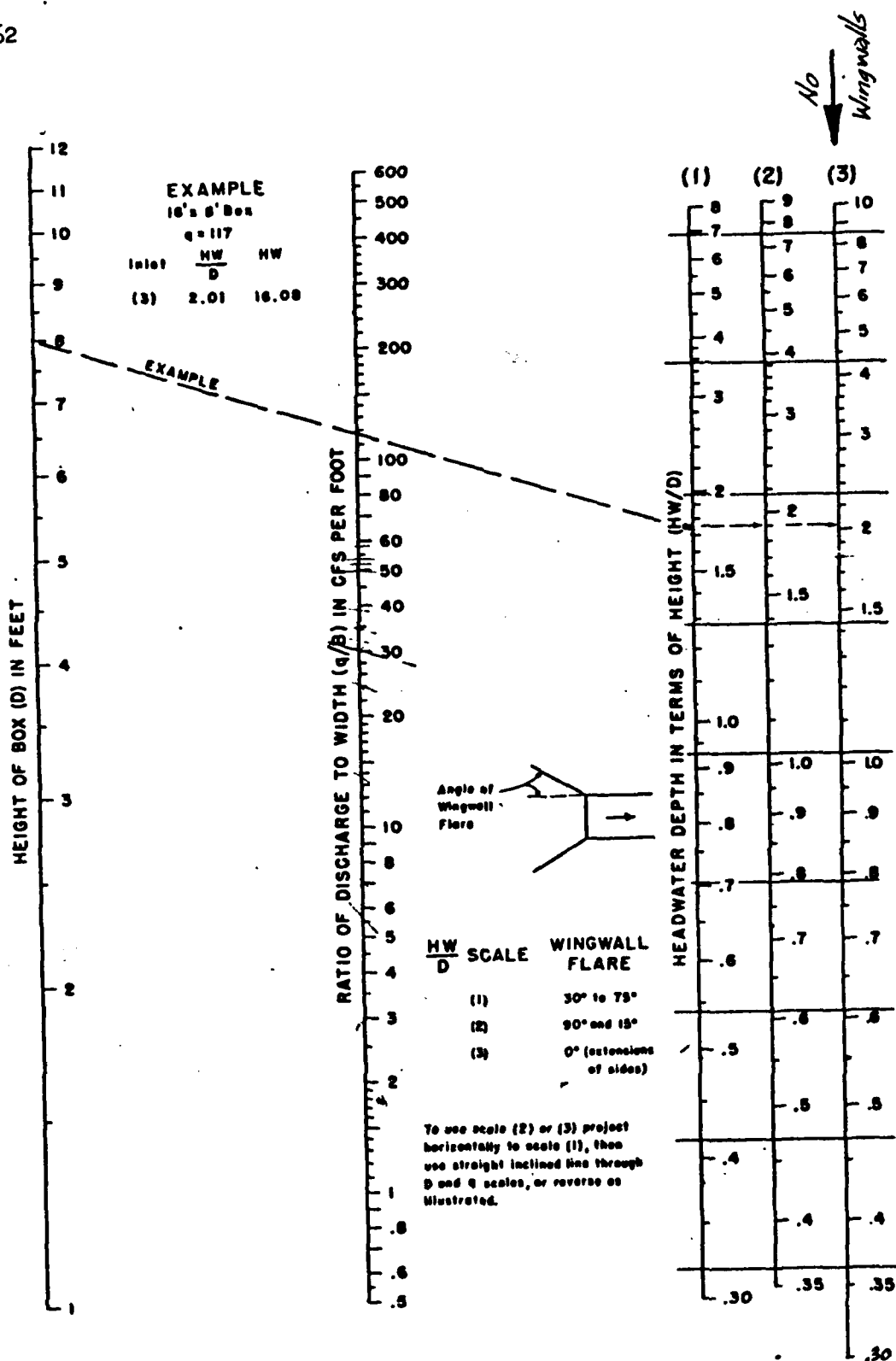
Spillway @ Mass. Rt. 2



Discharge thru Triple Culverts

Assume inlet control because culverts outlet into marsh area at least 1500' wide
 $D=5'$, $B=48'$ Refer to Ch. 14.6, Pg 14.62, SCS, NCH 4

ELV.	HW (ft)	HW/D	B/B	Q (cfs)	Total Q values	
832.5	1.5	0.30	7	330	ELV.	Q (cfs)
833.5	2.5	0.50	14	670	831.0	0
835.0	4.0	0.80	24	1150	832.5	330
837.0	6.0	1.20	39	1870	833.5	670
839.0 road surface = 8.0		1.60	50	2400	835.0	1150
840.0	7.0	1.80	54	2570	837.0	1870
841.0	10.0	2.0	58	2780	839.0	2400
Discharge over Top of Highway Emb. $Q = CLH^{3/2}$, $C=2.8$					840.0	3990
$C.E. 840.0$ $H=1'$, $L=500'$, $Q = 2.8 \cdot 500 \cdot 1^{3/2} = 1400$ cfs					841.0	7530
$C.E. 841.0$ $H=2'$, $L=600'$, $Q = 2.8 \cdot 600 \cdot 2^{3/2} = 4750$						



BUREAU OF PUBLIC ROADS JAN. 1963

Exhibit 14.6. Headwater depth for box culverts with inlet control.

From: Soil Conservation Service
National Engineering Hdbk 4

4

NATIONAL DAM SAFETY PROGRAM
NEW ENGLAND DIVISION - CORPS OF ENGINEERS

NO. 300 NHR 0 NMN 10 IDAY 0 IHR 0 IMIN 0 METRC 0 IFLT 0 IPRT -4 NSTAN 0
JOBER 5 NUT 0 LROPT 0 TRACE 0

RTIOS= .20 .30 .40 .50 .60 .70 .80 .90 1.00

SUB-AREA RUNOFF COMPUTATION

INFLOW TO LAKE ELLIS

ISTAG ICOMP IECON ITAPE JPLT JPRT INAME ISTAGE IAUTO
INFLOW 0 0 0 0 0 0 1 0 0

IHYUG IUNG TAKEA SNAP TRSDA TRSFC RATIO ISNDW ISAME LOCAL
1 1 3.00 0.00 3.00 0.00 0.00 0.00 0 1 0

PRECIP DATA

SPEE PMS R6 R12 R24 R48 R72 R96
0.00 20.50 111.00 123.00 133.00 142.00 0.00 0.00

TRSPC COMPUTED BY THE PROGRAM IS .800

LOSS DATA

LROPT STRKR DLTKR RTIOL ERAIN STRKS RTIOK STRTL CNSTL ALSMX RIIMP
0 0.00 0.00 1.00 0.00 0.00 1.00 0.00 .05 0.00 0.00

UNIT HYDROGRAPH DATA

TP= 3.60 CP=.60 NTA= 0

RECESSION DATA

STRTD= -1.70 ORCSN= -.10 RTIOR= 2.00

UNIT HYDROGRAPH100 END-OF-PERIOD ORDINATES, LAG= 3.58 HOURS, CP= .61 VOL= .98

3.	13.	27.	43.	61.	81.	103.	125.	148.	172.
196.	221.	245.	266.	284.	300.	313.	324.	332.	337.
339.	338.	332.	321.	306.	292.	279.	266.	254.	242.
231.	220.	210.	192.	174.	159.	142.	125.	109.	95.
145.	138.	132.	126.	120.	114.	109.	104.	99.	95.
90.	86.	82.	79.	75.	72.	68.	65.	62.	59.
57.	54.	52.	49.	47.	45.	43.	41.	39.	37.
35.	34.	32.	31.	29.	28.	27.	26.	24.	23.
22.	21.	20.	19.	18.	18.	17.	16.	15.	15.
14.	13.	13.	12.	12.	11.	10.	10.	10.	9.

MO.DA HR.MN PERIOD RAIN EXCS LOSS END-OF-PERIOD FLOW MO.DA HR.MN PERIOD RAIN EXCS LOSS COMP 0

SUM 23.29 21.54 1.74 222240.
(592.)(547.)(44.)(6434.72)

HYDROGRAPH ROUTING

ROUTED OUTFLOW FROM LAKE ELLIS DAM

ISTAQ	ICOMP	IECON	ITAPE	JPLT	JPRT	INAME	ISTAGE	IAUTO
OUTFLOW	1	0	0	0	0	1	0	0

ROUTING DATA

QLOSS	CLOSS	AUG	IRCS	ISAME	IOPT	IPMP	LSTR
0.0	0.000	0.00	1	1	0	0	0

NSTPS	NSTDL	LAG	AMSKK	X	TSK	STORA	ISPRAT
1	0	0	0.000	0.000	0.000	-831.	-1

STAGE	831.00	832.50	833.50	835.00	837.00	839.00	840.00
FLOW	0.00	330.00	670.00	1150.00	1870.00	2400.00	3990.00

SURFACE AREA= 0. 66. 140. 152.

CAPACITY= 0. 264. 969. 1115.

ELEVATION= 820. 832. 839. 840.

CREL	SPWID	COGW	EXPW	ELEV	COQL	CAREA	EXPL
832.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0

DAM DATA

TOPEL	COOD	EXPD	DAMWID
839.0	0.0	0.0	0.

PEAK OUTFLOW IS 724. AT TIME 45.17 HOURS

PEAK OUTFLOW IS 1064. AT TIME 45.17 HOURS

PEAK OUTFLOW IS 1406. AT TIME 45.17 HOURS

PEAK OUTFLOW IS 1734. AT TIME 45.33 HOURS

PEAK OUTFLOW IS 2009. AT TIME 45.50 HOURS

PEAK OUTFLOW IS 2252. AT TIME 45.67 HOURS

PEAK OUTFLOW IS 2738. AT TIME 45.33 HOURS

PEAK OUTFLOW IS 3389. AT TIME 45.00 HOURS

PEAK OUTFLOW IS 3952. AT TIME 44.67 HOURS

1

PEAK FLOW AND STORAGE (END OF PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, STORAGE IN CUBIC FEET PER SECOND

6

OPERATION	STATION	AREA	PLAN	RATIO 1	RATIO 2	RATIOS APPLIED TO FLOWS							
						RATIO 3	RATIO 4	RATIO 5	RATIO 6	RATIO 7	RATIO 8	RATIO 9	
				.20	.30	.40	.50	.60	.70	.80	.90	1.00	
HYDROGRAPH AT INFLOW	(3.00	1	982.	1473.	1964.	2455.	2946.	3437.	3928.	4419.	4910.	
	((7.77)	((27.81)	(41.71)	(55.62)	(69.52)	(83.42)	(97.33)	(111.23)	(125.14)	(139.04)	
ROUTED TO OUTFLO	(3.00	1	724.	1064.	1406.	1734.	2009.	2252.	2738.	3389.	3952.	
	((7.77)	((20.50)	(30.14)	(39.83)	(49.11)	(56.88)	(63.77)	(77.53)	(95.95)	(111.92)	

SUMMARY OF DAM SAFETY ANALYSIS

PLAN 1	ELEVATION	INITIAL VALUE	SPILLWAY CREST	TOP OF DAM
	STORAGE	831.00	832.00	839.00
	OUTFLOW	203.	264.	969.
		0.	220.	2400.

RATIO OF PMF	MAXIMUM RESERVOIR W.S.ELEV	MAXIMUM DEPTH OVER DAM	MAXIMUM STORAGE AC-FT	MAXIMUM OUTFLOW CFS	DURATION OVER TOP HOURS	TIME OF MAX OUTFLOW HOURS	TIME OF FAILURE HOURS
.20	833.67	0.00	387.	724.	0.00	45.17	0.00
.30	834.73	0.00	478.	1064.	0.00	45.17	0.00
.40	835.71	0.00	573.	1406.	0.00	45.17	0.00
.50	836.62	0.00	670.	1734.	0.00	45.33	0.00
.60	837.52	0.00	776.	2009.	0.00	45.50	0.00
.70	838.44	0.00	893.	2252.	0.00	45.67	0.00
.80	839.21	.21	999.	2738.	2.17	45.33	0.00
.90	839.62	.62	1058.	3389.	3.83	45.00	0.00
1.00	839.98	.98	1111.	3952.	4.83	44.67	0.00

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FLOOD HYDROGRAPH PACKAGE (HEC-1)

DAM SAFETY VERSION JULY 1978

LAST MODIFICATION 01 APR 80

EOI ENCOUNTERED.

C>BYE

JOB PROCESSING CCUS 3.464

BYE 81/02/25. 12.23.27.

select desired service:

END

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